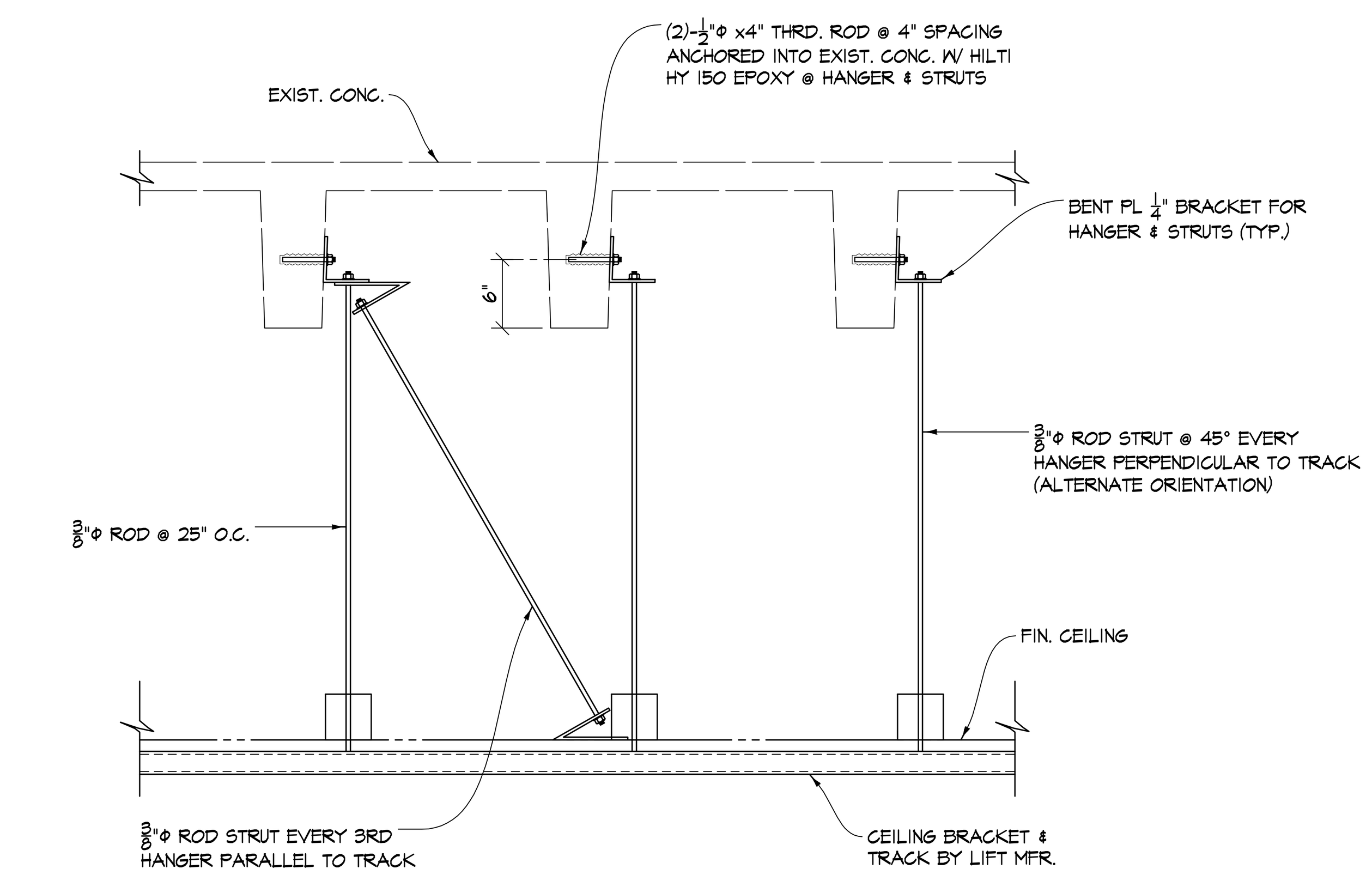
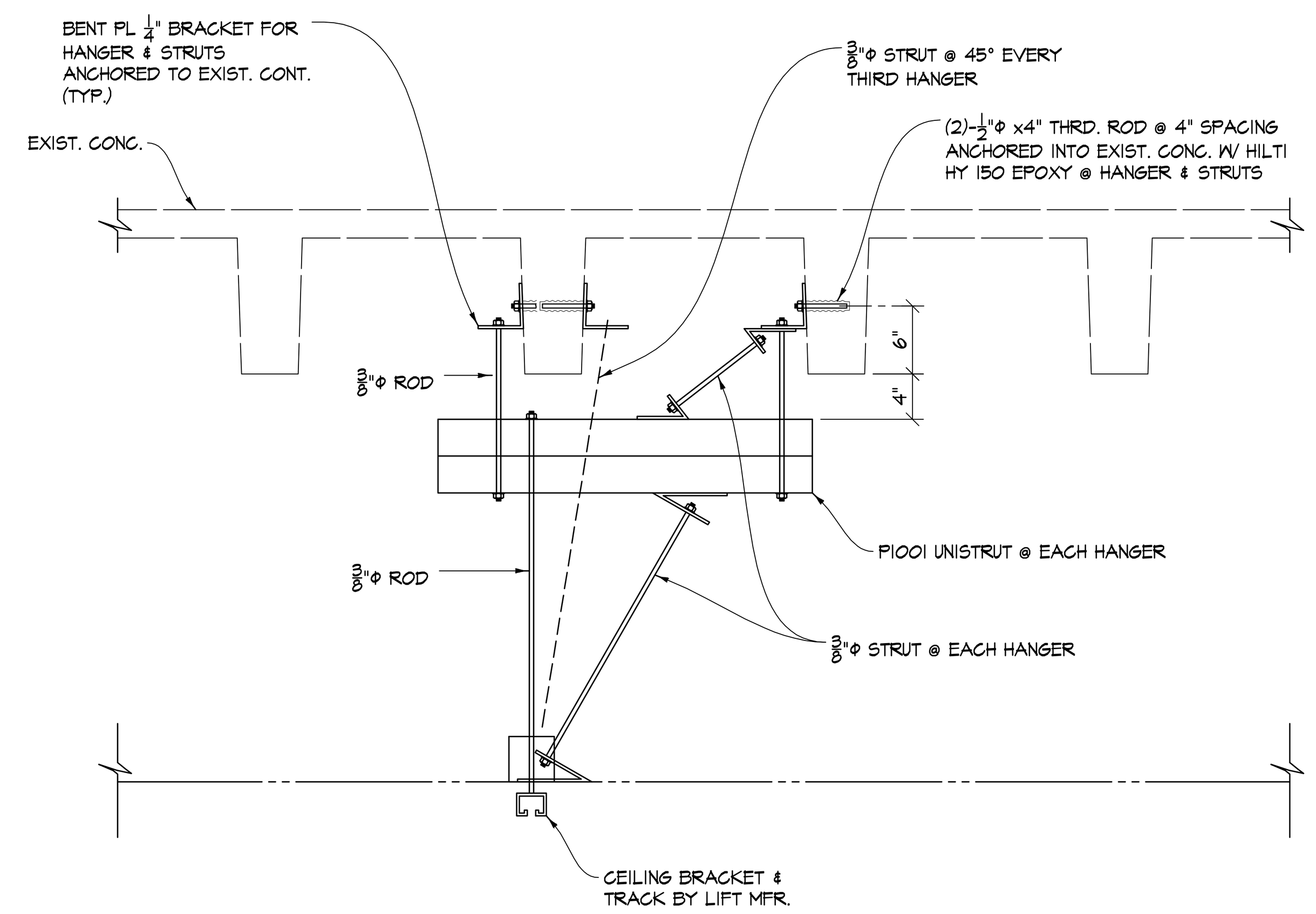
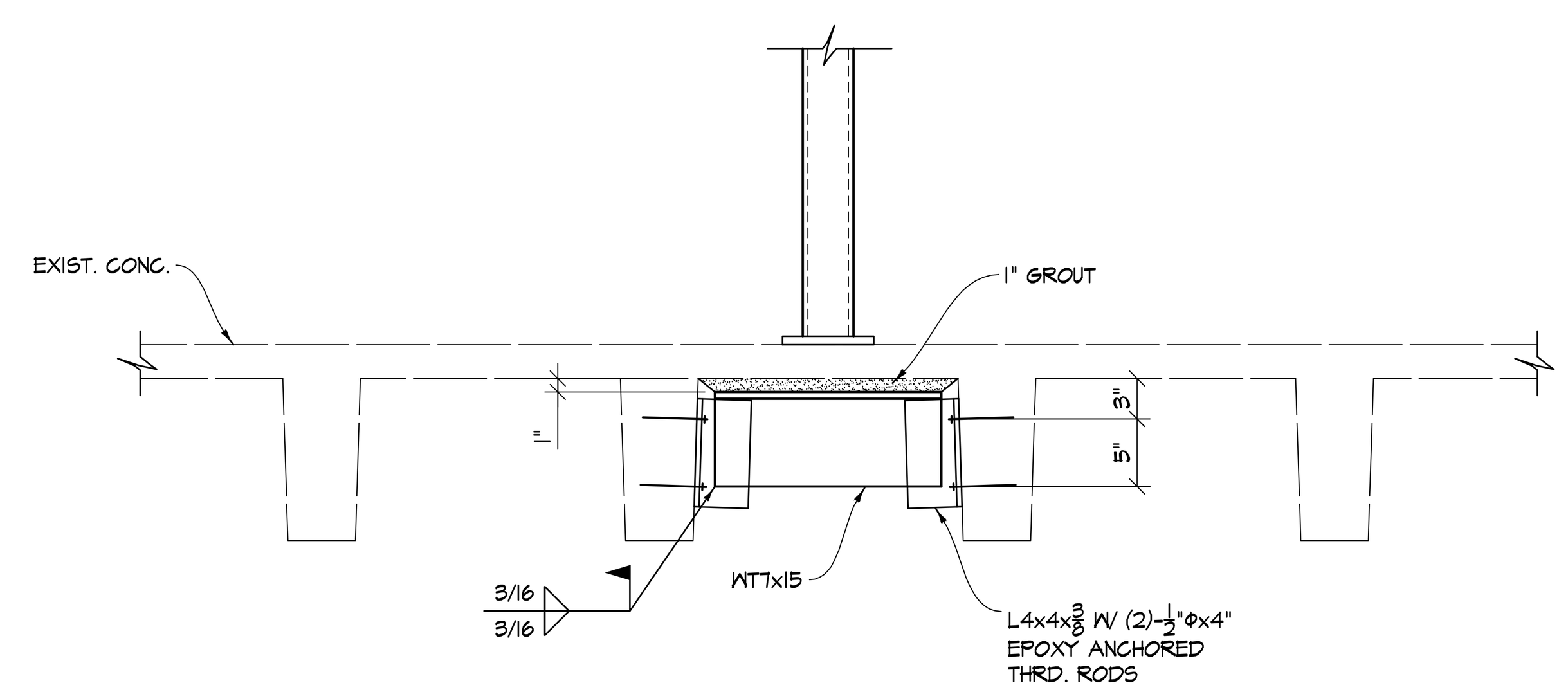




TYPICAL PATIENT LIFT TRACK SUPPORT DETAILS



TYPICAL SLAB REINF. @ COLUMNS OR BRACE CONNECTOR

GENERAL NOTES:

GENERAL:

1. THE CONTRACTOR SHALL VERIFY FIELD DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION AND NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR INCONSISTENCIES IN THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.
2. THESE DRAWINGS ARE NOT TO BE SCALED. CALLED DIMENSIONS GOVERN SCALED DIMENSIONS.
3. THE CONTRACTOR SHALL VERIFY LOCATIONS AND SIZES OF ALL OPENINGS IN FLOORS AND ROOFS AND ALL INSERTS AND EMBEDDED ITEMS WITH MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS BEFORE PLACING CONCRETE, INSTALLING METAL DECKING OR ERECTING ANY STRUCTURAL LOAD BEARING MATERIAL. THE GENERAL CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR ALL COORDINATION WITH SUB-CONTRACTORS.
4. ADEQUATE TEMPORARY BRACING WILL BE REQUIRED OF ALL STRUCTURAL PIECES OR UNITS UNTIL THE FLOOR AND ROOF DECK ARE IN PLACE, AND ALL CONCRETE HAS BEEN PLACED AND GAINED ITS ULTIMATE STRENGTH.
5. IN CASE OF DRAWING DISCREPANCIES IN DIMENSIONS AND ELEVATIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS, CONTRACTOR SHALL VERIFY WITH ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION.
6. THE LATEST EDITION OF IBC, ACI, AISC, AWS, CRSI, AND SJI SPECIFICATIONS SHALL GOVERN ALL PHASES OF FABRICATION AND CONSTRUCTION.
7. STRUCTURAL DESIGN CRITERIA:

   ROOF DEAD LOAD - 30 PSF
   ROOF LIVE LOAD - 20 PSF

   FLOOR LIVE LOAD - 100 PSF

   WIND LOAD - 90 MPH PER ASCE 7-05
   EXPOSURE C
   WIND IMPORTANCE FACTOR 1.15
   GCpi = +/- 0.18
   SEE CHART FOR COMPONENTS AND CLADDING DESIGN PRESSURES

   SEISMIC LOAD - PER IBC 2009
   SEISMIC IMPORTANCE FACTOR = 1.25
   OCCUPANCY CATEGORY II
   Ss = 0.16
   S1 = 0.07
   SITE CLASSIFICATION D
   Sds = 0.17
   Sd1 = 0.11
   SEISMIC DESIGN CATEGORY B
   SEISMIC FORCE RESISTING SYSTEM - ORDINARY CONCRETE SHEARWALLS
   RESPONSE MODIFICATION COEFFICIENT, R = 5
   SEISMIC RESPONSE COEFFICIENT, Cs = 0.05
   ANALYSIS PER EQUIVALENT LATERAL FORCE PROCEDURE
   DESIGN BASE SHEAR - IN = 1.4-KIPS
   IE = 0.5-KIPS

8. CONTRACTOR SHALL REVIEW AND STAMP ALL SUBMITTALS BEFORE FORWARDING TO ARCHITECT/ENGINEER FOR REVIEW. CONTRACTOR SHOULD INCORPORATE TEN (10) WORKING DAYS FOR ENGINEER REVIEW.

QUALITY ASSURANCE:

1. TESTING LABORATORY SHALL SUBMIT REPORTS INDICATING RESULTS AND OBSERVATIONS OF TESTS AND INSPECTIONS AND STATING COMPLIANCE OR NONCOMPLIANCE WITH CONTRACT DOCUMENTS TO ARCHITECT (STRUCTURAL ENGINEER) PER IBC 1705 AND 1706 AND GOVERNING CODE AUTHORITY. CONTRACTOR SHALL REIMBURSE OWNER FOR COSTS RELATED TO TESTS AND INSPECTIONS OF UNIDENTIFIABLE MATERIALS OR MATERIALS FURNISHED WITHOUT CERTIFIED LABORATORY TEST REPORTS, MATERIALS FOUND DEFICIENT AFTER INITIAL TESTS AND INSPECTIONS, OR MATERIALS REPLACING DEFICIENT MATERIALS. SEE SPECIFICATIONS FOR ADDITIONAL TEST AND INSPECTION REQUIREMENTS.
2. PROVIDE STRUCTURAL STEEL, HIGH-STRENGTH BOLTS, FROM IDENTIFIABLE TESTED STOCK. SUBMIT CERTIFIED LABORATORY TEST REPORTS TO ARCHITECT (STRUCTURAL ENGINEER) AND GOVERNING CODE AUTHORITY. IF MATERIALS CANNOT BE IDENTIFIED OR IF CERTIFIED LABORATORY TEST REPORTS CANNOT BE MADE AVAILABLE, TESTING LABORATORY WILL PERFORM TESTS TO DETERMINE CONFORMANCE WITH CONTRACT DOCUMENTS AS DIRECTED BY ARCHITECT (STRUCTURAL ENGINEER).
3. TESTING LABORATORY SHALL PROVIDE SPECIAL INSPECTIONS, COMPLYING WITH IBC SECTION 1704 FOR THE FOLLOWING:
   a. STEEL CONSTRUCTION PER TABLE 1704.3
   b. CONCRETE CONSTRUCTION PER TABLE 1704.4
   c. SPRAYED FIRE-RESISTANT MATERIALS PER 1704.12.
4. SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE PER 1707 ARE NOT REQUIRED FOR THIS PROJECT.

CONCRETE:

1. CONCRETE SHALL HAVE THE FOLLOWING 28-DAY STRENGTHS:
   FOOTINGS, GRADE BEAMS, AND SLABS-ON-GRADE - 3,000 PSI

   AIR ENTRAINMENT SHALL NOT EXCEED 3% FOR INTERIOR SLABS WITH A STEEL TROWEL FINISH.
2. CHAMFER ALL EXPOSED EDGES TO 3/4" UNLESS OTHERWISE NOTED.
3. MAXIMUM AGGREGATE SHALL BE AS FOLLOWS:
   FOOTINGS, GRADE BEAMS, AND SLABS-ON-GRADE - 1-1/2"
4. ALL CONSTRUCTION JOINTS IN BEAMS SHALL BE KEYED USING A HORIZONTAL KEYWAY AND JOINT SHALL BE LOCATED AT MIDDLE THIRD OF SPAN. REINFORCING SHALL BE CONTINUOUS THROUGH JOINT.
5. GRIND ALL CONSTRUCTION JOINTS IN SLABS SO AS TO PRODUCE A SMOOTH AND LEVEL SURFACE.
6. PLACE SLABS-ON-GRADE IN ALTERNATE PANELS SEPARATED BY CONSTRUCTION JOINTS (C.J.) OR STRIP POUR AND SEPARATE BY CONTRACTION JOINTS (D.J.) MADE BY SAW CUTTING OR INSERTING PREFORMED HARDBOARD TO A MINIMUM DEPTH OF 1/4 SLAB DEPTH. EACH PANEL MAY NOT EXCEED 225 SQUARE FEET IN AREA AND SIDES MAY NOT EXCEED 1.5:1.0 RATIO. IF JOINT PATTERN IS NOT SHOWN ON THE DRAWINGS, CONTRACTOR SHALL SUBMIT JOINT LAYOUT AND OBTAIN APPROVAL OF ARCHITECT BEFORE BEGINNING SLAB-ON-GRADE CONSTRUCTION.
7. ALL EXPOSED CONCRETE TO RECEIVE RUBBED FINISH UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL:

2. ALL STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE LATEST EDITION OF THE AISC MANUAL OF STEEL CONSTRUCTION "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - ALLOWABLE STRESS DESIGN." STEEL FOR WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992 (50 KSI); RECTANGULAR HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500 GR. B (46 KSI); ROUND HOLLOW STRUCTURAL SECTIONS SHALL CONFORM TO ASTM A500 GR. B (42 KSI); PIPES SHALL CONFORM TO ASTM A53 GR. B (35 KSI); ALL OTHER STRUCTURAL STEEL SHAPES AND PLATE SHALL CONFORM TO ASTM A36 (36 KSI).
3. CONNECTIONS OF STRUCTURAL STEEL BEAMS TO COLUMNS AND BEAMS TO BEAMS SHALL BE BEARING TYPE WITH STANDARD CLIP ANGLES WITH THE MAXIMUM NUMBER OF BOLT ROWS USING 3" SPACING PERMITTED BY THE INDIVIDUAL BEAM SIZE, UNLESS NOTED OTHERWISE. STRUCTURAL STEEL FLOOR OR ROOF FILLER BEAMS CARRYING GRAVITY LOAD ONLY MAY BE CONNECTED TO GIRDERS USING SINGLE ANGLE SHEAR CONNECTIONS SIZED TO DEVELOP THE DESIGN REACTIONS SHOWN.
4. ALL WELDING SHALL CONFORM TO THE RECOMMENDATIONS OF THE AWS, AND ALL WELDS, INCLUDING FIELD, SHALL BE MADE ONLY BY CERTIFIED WELDERS USING E70XX ELECTRODES.
5. ALL STRUCTURAL BOLTS SHALL BE 3/4"-DIAMETER ASTM A325-N, BEARING TYPE UNLESS NOTED OTHERWISE.
6. ALL ROOF OPENINGS SMALLER THAN 12" IN PLAN SHALL BE FRAMED WITH L 3x3x1/4 ANGLES ON ALL SIDES. OPENINGS LARGER THAN 12" IN PLAN SHALL BE FRAMED AS REQUIRED TO CARRY THE LOADS APPLIED.
7. ROOF DECKING SHALL BE 1-1/2", 22 GAGE, TYPE B, GALVANIZED STEEL DECK. ATTACH ROOF DECK TO SUPPORTS WITH 5/8" PUDDLE WELDS AT 12" O.C. (36/4 FASTENER PATTERN). PROVIDE ONE (1) #10 TEK SCREW SIDE LAP FASTENER AT MIDSPAN OF DECK, TYPICAL UNLESS NOTED OTHERWISE. ATTACH ROOF DECK TO PERIMETER SUPPORTS WITH 5/8" PUDDLE WELDS AT 6" O.C. AT ROOF EDGES.
8. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL ITEMS NOT SHOWN ON STRUCTURAL DRAWINGS.
9. CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS AND OBTAIN APPROVAL PRIOR TO FABRICATION.

MISCELLANEOUS STEEL:

REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MISCELLANEOUS STEEL ITEMS.

LIGHT GAGE STEEL TRUSSES AND LOAD-BEARING FRAMING:

1. LOCATE LOAD-BEARING STUDS DIRECTLY UNDER JOIST BEARING POINTS.
2. LIGHT GAGE STEEL ROOF FRAMING MEMBER CONNECTIONS SHALL BE DESIGNED BY SUPPLIER TO SUPPORT THE LOADS DESIGNATED.
3. ROOF JOIST FRAMING CONNECTIONS SHALL BE DESIGNED FOR THE FOLLOWING LOADS:
   ROOF DEAD LOAD - 30 PSF
   ROOF LIVE LOAD - 20 PSF
4. CALCULATIONS AND CONNECTION DETAILS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF LOUISIANA SHALL BE SUBMITTED FOR APPROVAL PRIOR TO FABRICATION OF LIGHT GAGE COMPONENTS.

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THESE DRAWINGS DO NOT INCLUDE NECESSARY COMPONENTS FOR CONSTRUCTION SAFETY. 10.117

Scale: AS NOTED

Revisions:	Date

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STATE OF LOUISIANA — MATTHEW J. WALLACE — License No. 25922 — PROFESSIONAL ENGINEER — CIVIL ENGINEERING — 12/18/12

PROJECT MANAGERS AND ENGINEERS:

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SOUTH BARRINGTON, IL 60010

Drawing Title: GENERAL NOTES

Approved: Project Director

Project Title: RENOVATE ED & PRIMARY CARE

Location: SHREVEPORT VAMC, 510 EAST STONER AVE

Date: 01/09/13 | Checked: M.J.W. | Drawn: P.S.F.

Project Number: 667-10-100

Building Number: 1N, 1E

Drawing Number: S2.1

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Office of Construction and Facilities Management
Department of Veterans Affairs

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